



Performance Analysis Of Tugu Pancasila Roundabout In Putussibau City Kapuas Hulu District

Hifzhan Hanif Darmawan¹, Elsa Tri Mukti², and S. Nurlaili Kadarini³

¹Department of Civil Engineering, University of Tanjungpura
 hanifdarmawan511@gmail.com

ABSTRACT	ARTICLE INFO
<p>Tugu Pancasila Roundabout in Putussibau City is a 5-arm roundabout that organizes traffic in 5 directions of movement. Poor traffic management at this roundabout, such as the lack of roundabout signs and no separation island or median on roads other than the main road, makes motorists who should pass through the roundabout instead choose not to cross the roundabout and go against the flow because they want to reach their destination faster. In addition, due to the increasing population and the volume of vehicles traveling, it is anticipated that in the future, the Tugu Pancasila roundabout will have increasingly heavy traffic flows. The performance analysis of Tugu Pancasila Roundabout was calculated using MKJI 1997 and VISSIM software. The performance analysis of Tugu Pancasila Roundabout was carried out by taking roundabout geometric data, manually taking side obstacle data, and traffic volume data for three days using survey aids in the form of CCTV starting at 06.00 - 18.00 WIB. Analysis of the existing conditions in 2023, the highest degree of saturation is 0.62 with the level of service B; in the 2028 projection conditions, the highest degree of saturation is 0.73 with the level of service B; and in the 2033 projection conditions, the highest degree of saturation is 0.86 with the level of service E. With a poor level of service, the volume of vehicles and the roundabout capacity could be more balanced, so improvements are needed for the Tugu Pancasila Roundabout in Putussibau City. The improvement solution is to re-plan the roundabout following the Roundabout Planning Guidelines for Intersections (2004). For the results of the MKJI 1997 analysis of the roundabout re-planning solution, the 2028 projection obtained the highest degree of saturation of 0.54 with level of service B, and in the 2033 projection obtained the highest degree of saturation of 0.63 with level of service B. for the results of the VISSIM software simulation, the roundabout re-planning solution for the 2028 projection obtained a delay of 13.14 seconds with level of service B, and the 2033 projection obtained a delay of 15.60 seconds with level of service C.</p> <p>Keywords: <i>Roundabout, Performance Analysis, Putussibau</i></p>	<p>* Corresponding Author hanifdarmawan511@gmail.com</p> <p>Citation: Darmawan, H., H.; Mukti, E., T.; Kadarini, S., N. (2024). Performance Analysis Of Tugu Pancasila Roundabout In Putussibau City Kapuas Hulu District. Jurnal Teknik Sipil (JTS) Vol. 24, 1. p.677-689. https://doi.org/10.26418/jts.v24i1.75946</p> <p>Submitted: 28-Jan-2024 Accepted: 28-Feb-2024 Revised: 10-Mar-2024 Published: 17-Mar-2024</p> <p>Publisher's Note: JTS stays neutral about jurisdictional claims in published maps and institutional affiliations</p>

1. Introduction

An intersection can be defined as the general area where 2 or more roads join or intersect, including the road and roadside facilities for the movement of traffic therein (Khisty and Lall, 2005; Lo et al., 2023). Roundabout is a type of directional traffic intersection control that circles an island in the

centre of the intersection. Roundabout intersection planning is a very important part of highway planning. At roundabouts, there is a meeting between vehicles from various directions (Osei et al., 2021; Sukmawati, et. al., 2022).

Tugu Pancasila Roundabout is a 5-arm intersection roundabout that regulates traffic in 5 directions of movement. Connecting office areas, educational institutions, residential areas, and economic areas, as well as serving traffic flows in and out of the city to the north, makes this roundabout possible for conflict flows because it serves essential flows.

In this research, the Tugu Pancasila Roundabout is modeled using VISSIM software. VISSIM (Verkher In Stadten Simulation Model) software is a multi-modal traffic flow microscopic simulation software that can analyze the operation of private vehicles and public transport with problems such as lane configuration, vehicle composition, traffic signals, and others, making VISSIM a valuable tool for the evaluation of various alternative measures based on transportation engineering measures and planning effectiveness (PTV VISION, 2011).

2. Materials and Methods

2.1. Study Area/ Research Location

This research was conducted at the Tugu Pancasila Roundabout which connects D. I. Panjaitan Street, W. R. Supratman Street, Gajah Mada Street, Pancasila Street, and Jerending Abdurahman Street.



Fig 1. Research Location

2.2. Data

The data used in the research are primary data and secondary data. Primary data is data obtained or collected by researchers directly from the source of the data or in the form of original data (Siyoto and Sodik, et. al., 2015). The following primary data is needed in this study:

- Geometrics Data
- Traffic Volume Data
- Vehicle Speed Data
- Side Obstacle Data
- Data on Existing Buildings around the Roundabout

Secondary data is data obtained or collected from various existing sources, such as books, reports, journals, etc. The following secondary data is needed in this study:

- Research Location Map
- Total Population Data
- Total Vehicle Data

2.3. Analysis Method

Data analysis is carried out using the 1997 Indonesian Road Capacity Manual (MKJI) method and using VISSIM Software, then evaluated, then given a planning solution for the improvement of the Tugu Pancasila Roundabout in Putussibau City.

2.3.1 Capacity

According to MKJI 1997, the total capacity of the braided section of the roundabout is the product of the basic capacity (C_0), which is the capacity under certain (ideal) conditions and the adjustment factor (F), taking into account the effect of actual field conditions on capacity.

$$C = 135 \times WW^{1,3} \times (1 + WE/WW)^{1,5} \times (1 + PW/3)^{0,5} \times (1 + WW/LW) - 1,8 \times FCS \times FRSU \quad (1)$$

2.3.2 Degree of Saturation

According to MKJI 1997, the degree of saturation is the ratio of traffic flow to capacity, which is usually calculated per hour. The traffic behaviour of the braided section is closely related to the degree of saturation.

$$DS = \frac{Q_{SMP}}{C} \quad (2)$$

2.3.3 Delay

According to MKJI 1997, delays in the braided section can occur due to traffic delays (DT) that result from the interaction of traffic with other movements in the intersection, and geometric delays (DG) that result from the deceleration and acceleration of traffic.

$$D = DT + DG \quad (3)$$

$$DG = (1 - DS) \times 4 + DS \quad (4)$$

$$DT_R = \sum \frac{(Q_i \times DT_i)}{Q_{in}}; i = 1 \dots n \quad (5)$$

$$DR = DT_R + 4 \quad (6)$$

2.3.4 Queuing Opportunities

The chance of queuing with more than 2 vehicles on any approach, at an unsignalised intersection. This queuing probability is calculated from the empirical relationship between queuing probability and the degree of saturation.

$$QPR\% = maks(QPi\%); i = 1 \dots n \quad (7)$$

2.3.5 Level of Service

Level of service are categorised from A to F with level of service A representing the best service operating conditions and level of service F representing the worst service operating conditions.

Table 1. Level of Service based on Degree of Saturation

Level of Service	Degree of Saturation
A	0,00 – 0,20
B	0,21 – 0,44
C	0,45 – 0,74
D	0,75 – 0,84
E	0,85 – 1,00
F	> 1,00

Table 2. Level of Service based on Delay

Level of Service	Delay (sec/smp)
A	0 – 5
B	5 – 15
C	15 – 25
D	25 – 40
E	40 – 60
F	> 60

3. Result and Discussion

3.1 Performance Analysis of Roundabout in Existing Conditions in 2023

Traffic performance analysis of roundabouts in existing conditions is carried out based on primary data processing of road geometric survey results, traffic volume, vehicle speed, and side obstacles on the braid section and road sections studied. Analysis of geometric parameter calculations, capacity calculations, and traffic behaviour of the braid section aims to determine the capacity value, degree of saturation, delay and queuing opportunities of the roundabout.

The geometric parameters of the roundabout braided section can be seen in Figure 2.

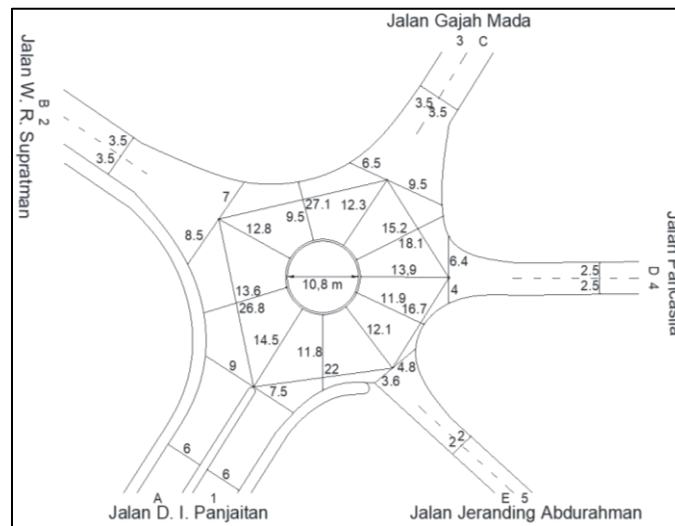


Fig 2. Braid Section Geometric Parameter of Roundabout in Existing Condition

The results of the braid section geometric parameters calculation of the existing condition can be seen in Table 3.

Table 3. Braid Section Geometric Parameter Calculation Results of The Existing Condition

No	Braid Section	Entry Width		Average Entry Width	Braid Width	WE/WW	Braid Length	WW/LW
		Approach 1 (m)	Approach 2 (m)	WE (m)	WW (m)		LW (m)	
1	AB	9	14,5	11,75	13,6	0,864	26,8	0,507
2	BC	7	12,8	9,9	9,5	1,042	27,1	0,351
3	CD	9,5	12,3	10,9	15,2	0,717	18,1	0,840
4	DE	4	13,9	8,95	11,9	0,752	16,7	0,713
5	EA	3,6	12,1	7,85	11,8	0,665	22	0,536

The results of the braid section capacity calculation of the existing condition can be seen in Table 4.

Table 4. Braid Section Capacity Calculation Results of The Existing Condition

No	Braid Section	WW Factor	WE/WW Factor	PW Factor	WW/LW Factor	Base Capacity C0	Adjustment Factor		Capacity C smp/hour
							City Size	Street Environment (FRSU)	
1	AB	4017,301	2,545	0,523	0,478	2555,606	0,88	0,95	2136,486
2	BC	2519,849	2,918	0,526	0,582	2253,120	0,88	0,95	1883,608
3	CD	4642,270	2,250	0,558	0,334	1943,988	0,88	0,95	1625,174
4	DE	3377,107	2,319	0,682	0,380	2028,663	0,88	0,95	1695,962
5	EA	3340,261	2,149	0,449	0,462	1488,790	0,88	0,95	1244,628

The results of the braid section traffic behaviour calculation for the existing condition can be seen in Table 5.

Table 5. Braid Section Traffic Behaviour Calculation Results of The Existing Condition

No	Braid Section	Flow of Braid Section Q smp/hour	Degree of Saturation DS	Traffic Delay DT sec/smp	Total Traffic Delay DTOT = Q x DT	Queue Opportuniy QP%
1	AB	727	0,34	1,6	1160,498	3 - 7
2	BC	781	0,41	1,9	1517,518	5 - 9
3	CD	740	0,46	2,1	1578,520	5 - 11
4	DE	714	0,42	2,0	1410,127	5 - 9
5	EA	766	0,62	3,0	2258,382	10 - 21
6	Degree of Saturation (DS)					0,62
7	Average Traffic Delay (DTR) sec/smp					5,8
8	Average Roundabout Delay DR (DTR+4) sec/smp					9,78
9	Roundabout Queue Opportunity QPR%					10 - 21

The results of the VISSIM software simulation of existing conditions can be seen in Table 6.

Table 6. VISSIM Simulation Results of The Existing Condition

Code	From	Towards	Queue Length (m)	Maximum Queue Length (m)	Delay (sec)	Level of Service
A1	D. I. Panjaitan	D. I. Panjaitan	4,02	26,66	13,10	B
A2	D. I. Panjaitan	W. R. Supratman	4,02	26,66	8,21	A
A3	D. I. Panjaitan	Gajah Mada	4,02	26,66	13,42	B
A4	D. I. Panjaitan	Pancasila	4,02	26,66	7,07	A
A5	D. I. Panjaitan	Jeranding Abdurahman	4,02	26,66	7,51	A
B1	W. R. Supratman	D. I. Panjaitan	2,67	17,82	15,40	C
B2	W. R. Supratman	W. R. Supratman	2,67	17,82	0,00	A
B3	W. R. Supratman	Gajah Mada	2,67	17,82	9,97	A
B4	W. R. Supratman	Pancasila	2,67	17,82	17,84	C
B5	W. R. Supratman	Jeranding Abdurahman	2,67	17,82	17,41	C
C1	Gajah Mada	D. I. Panjaitan	33,54	114,94	40,91	E
C2	Gajah Mada	W. R. Supratman	33,54	114,94	13,63	B
C3	Gajah Mada	Gajah Mada	33,54	114,94	0,00	A
C4	Gajah Mada	Pancasila	33,54	114,94	7,75	A
C5	Gajah Mada	Jeranding Abdurahman	33,54	114,94	35,18	E
D1	Pancasila	D. I. Panjaitan	4,69	39,45	13,85	B
D2	Pancasila	W. R. Supratman	4,69	39,45	13,19	B
D3	Pancasila	Gajah Mada	4,69	39,45	11,57	B
D4	Pancasila	Pancasila	4,69	39,45	0,00	A
D5	Pancasila	Jeranding Abdurahman	4,69	39,45	14,32	B
E1	Jeranding Abdurahman	D. I. Panjaitan	15,93	82,44	29,82	D
E2	Jeranding Abdurahman	W. R. Supratman	15,93	82,44	31,66	D
E3	Jeranding Abdurahman	Gajah Mada	15,93	82,44	26,32	D
E4	Jeranding Abdurahman	Pancasila	15,93	82,44	39,57	E
E5	Jeranding Abdurahman	Jeranding Abdurahman	15,93	82,44	0,00	A
Roundabout			12,17	114,94	17,81	C

3.2 Performance Analysis of Roundabout in Projection Conditions for 2028

Traffic performance analysis of the roundabout under the 2028 projection conditions, to determine the performance of the roundabout in 2028. This analysis uses the compound interest formula to project the peak hour volumes that have been analysed.

The results of the braid section traffic behaviour calculation of the 2028 projection conditions can be seen in Table 7.

Table 7. Braid Section Traffic Behaviour Calculation Results of The 2028 Projection Condition

No	Braid Section	Flow of Braid Section Q smp/hour	Degree of Saturation DS	Traffic Delay DT sec/smp	Total Traffic Delay DTOT = Q x DT	Queue Opportuniy QP%
1	AB	860	0,40	1,9	1624,220	4 - 9
2	BC	924	0,49	2,3	2124,802	6 - 13
3	CD	875	0,54	2,5	2209,744	7 - 15
4	DE	845	0,50	2,3	1975,968	6 - 13
5	EA	906	0,73	4,2	3830,209	15 - 32
6	Degree of Saturation (DS)					0,73
7	Average Traffic Delay (DTR) sec/smp					7,2
8	Average Roundabout Delay DR (DTR+4) sec/smp					11,25
9	Roundabout Queue Opportuniy QPR%					15 - 32

The results of the VISSIM software simulation of the 2028 projection conditions can be seen in Table 8.

Table 8. VISSIM Simulation Results of The 2028 Projection Condition

Code	From	Towards	Queue Length (m)	Maximum Queue Length (m)	Delay (sec)	Level of Service
A1	D. I. Panjaitan	D. I. Panjaitan	10,61	63,68	27,25	D
A2	D. I. Panjaitan	W. R. Supratman	10,61	63,68	18,23	C
A3	D. I. Panjaitan	Gajah Mada	10,61	63,68	24,34	C
A4	D. I. Panjaitan	Pancasila	10,61	63,68	21,77	C
A5	D. I. Panjaitan	Jeranding Abdurahman	10,61	63,68	23,00	C
B1	W. R. Supratman	D. I. Panjaitan	2,02	17,65	18,43	C
B2	W. R. Supratman	W. R. Supratman	2,02	17,65	0,00	A
B3	W. R. Supratman	Gajah Mada	2,02	17,65	11,48	B
B4	W. R. Supratman	Pancasila	2,02	17,65	16,03	C
B5	W. R. Supratman	Jeranding Abdurahman	2,02	17,65	20,23	C
C1	Gajah Mada	D. I. Panjaitan	13,68	61,22	31,23	D
C2	Gajah Mada	W. R. Supratman	13,68	61,22	35,83	E
C3	Gajah Mada	Gajah Mada	13,68	61,22	0,00	A
C4	Gajah Mada	Pancasila	13,68	61,22	14,19	B
C5	Gajah Mada	Jeranding Abdurahman	13,68	61,22	40,51	E
D1	Pancasila	D. I. Panjaitan	15,28	57,85	33,19	D
D2	Pancasila	W. R. Supratman	15,28	57,85	46,26	E
D3	Pancasila	Gajah Mada	15,28	57,85	26,49	D
D4	Pancasila	Pancasila	15,28	57,85	0,00	A
D5	Pancasila	Jeranding Abdurahman	15,28	57,85	25,07	D
E1	Jeranding Abdurahman	D. I. Panjaitan	50,70	182,76	49,07	E
E2	Jeranding Abdurahman	W. R. Supratman	50,70	182,76	55,52	F
E3	Jeranding Abdurahman	Gajah Mada	50,70	182,76	50,24	F
E4	Jeranding Abdurahman	Pancasila	50,70	182,76	79,40	F
E5	Jeranding Abdurahman	Jeranding Abdurahman	50,70	182,76	0,00	A
	Roundabout		18,46	182,76	28,21	D

3.3 Performance Analysis of Roundabout in Projection Conditions for 2033

Traffic performance analysis of the roundabout under the 2033 projection conditions, to determine the performance of the roundabout in 2033. This analysis uses the compound interest formula to project the peak hour volumes that have been analysed.

The results of the braid section traffic behaviour calculation of the 2033 projection conditions can be seen in Table 9.

Table 9. Braid Section Traffic Behaviour Calculation Results of The 2033 Projection Condition

No	Braid Section	Flow of Braid Section Q smp/hour	Degree of Saturation DS	Traffic Delay DT sec/smp	Total Traffic Delay DTOT = Q x DT	Queue Opportunily QP%
1	AB	1018	0,48	2,2	2276,514	6 - 12
2	BC	1094	0,58	2,7	2979,258	8 - 18
3	CD	1036	0,64	3,2	3279,772	10 - 23
4	DE	1001	0,59	2,8	2772,567	9 - 19
5	EA	1073	0,86	6,9	7415,125	24 - 51
6	Degree of Saturation (DS)					0,86
7	Average Traffic Delay (DTR) sec/smp					9,7
8	Average Roundabout Delay DR (DTR+4) sec/smp					13,75
9	Roundabout Queue Opportunity QPR%					24 - 51

The results of the VISSIM software simulation of the 2033 projection conditions can be seen in Table 10.

Table 10. VISSIM Simulation Results of The 2033 Projection Condition

Code	From	Towards	Queue Length (m)	Maximum Queue Length (m)	Delay (sec)	Level of Service
A1	D. I. Panjaitan	D. I. Panjaitan	21,17	53,56	28,14	D
A2	D. I. Panjaitan	W. R. Supratman	21,17	53,56	28,21	D
A3	D. I. Panjaitan	Gajah Mada	21,17	53,56	33,23	D
A4	D. I. Panjaitan	Pancasila	21,17	53,56	32,63	D
A5	D. I. Panjaitan	Jeranding Abdurahman	21,17	53,56	33,81	D
B1	W. R. Supratman	D. I. Panjaitan	27,45	131,37	50,76	F
B2	W. R. Supratman	W. R. Supratman	27,45	131,37	67,07	F
B3	W. R. Supratman	Gajah Mada	27,45	131,37	60,23	F
B4	W. R. Supratman	Pancasila	27,45	131,37	56,68	F
B5	W. R. Supratman	Jeranding Abdurahman	27,45	131,37	45,50	E
C1	Gajah Mada	D. I. Panjaitan	70,60	218,04	50,24	F
C2	Gajah Mada	W. R. Supratman	70,60	218,04	144,49	F
C3	Gajah Mada	Gajah Mada	70,60	218,04	0,00	A
C4	Gajah Mada	Pancasila	70,60	218,04	2,50	A
C5	Gajah Mada	Jeranding Abdurahman	70,60	218,04	39,45	E
D1	Pancasila	D. I. Panjaitan	26,50	106,65	44,04	E
D2	Pancasila	W. R. Supratman	26,50	106,65	47,64	E
D3	Pancasila	Gajah Mada	26,50	106,65	32,87	D
D4	Pancasila	Pancasila	26,50	106,65	0,00	A
D5	Pancasila	Jeranding Abdurahman	26,50	106,65	56,10	F
E1	Jeranding Abdurahman	D. I. Panjaitan	95,69	239,30	132,92	F
E2	Jeranding Abdurahman	W. R. Supratman	95,69	239,30	122,45	F
E3	Jeranding Abdurahman	Gajah Mada	95,69	239,30	123,30	F
E4	Jeranding Abdurahman	Pancasila	95,69	239,30	144,54	F
E5	Jeranding Abdurahman	Jeranding Abdurahman	95,69	239,30	0,00	A
	Roundabout		48,28	239,30	48,31	E

3.4 Classification of Performance Analysis Results of Existing Conditions, Projection Conditions in 2028 and 2033

The class classification of the highest degree of saturation and average delay of existing conditions in 2023, and projection conditions in 2028 and 2033, based on MKJI 1997 calculations can be seen in Table 11.

Table 11. Class Classification of Degree of Saturation and Average Delay of Existing Conditions, Projection Condition for 2028, and 2033

No	Classification	Result	Level of Service
1	DS _R Existing Result (2023)	0,62	C
	DS _R Projection Result (2028)	0,73	C
	DS _R Projection Result (2033)	0,86	E
2	DT _R (sec) Existing Result (2023)	9,78	B
	DT _R (sec) Projection Result (2028)	11,25	B
	DT _R (sec) Projection Result (2033)	13,75	B

After simulation in VISSIM software. Class classification on the value of delay based on simulation results for existing conditions in 2023, and projection conditions in 2028 and 2033, can be seen in Table 12.

Table 12. Class Classification of Delays Based on VISSIM Software Simulations of Existing Conditions, Projection Condition for 2028, and 2033

No	Classification	Result	Level of Service
1	Delay Existing Result (2023)	17,81	C
2	Delay Projection Result (2028)	28,21	D
3	Delay Projection Result (2033)	48,31	E

3.5 Improvement Solutions for Tugu Pancasila Roundabout Putussibau City

The performance of the Tugu Pancasila Roundabout in Putussibau City results in the existing condition in 2023 in the highest degree of saturation value of 0.62 with level of service C, for the projection condition in 2028 resulted in the highest degree of saturation value of 0.73 with level of service C, and for the projection condition in 2033 resulted in the highest degree of saturation value of 0.86 with level of service E. The performance of the Tugu Pancasila Roundabout in Putussibau City for the 2033 projection with a degree of saturation value of DS 0.85 - 1.00 resulted in a decrease in the level of performance at the roundabout such as the occurrence of high traffic density causing congestion. Based on the analysis results, it is necessary to have an improvement solution in the future for the Tugu Pancasila Roundabout in Putussibau City, to reduce density and traffic violations so that traffic movements reach a good level of service where the final result gets a DS value <0.75. The improvement solution to be carried out is to re-plan the roundabout following the Roundabout Planning Guidelines for Level Crossing (2004).

3.6 Tugu Pancasila Roundabout Re-planning Solutions

The solution attempted at the Tugu Pancasila roundabout in Putussibau City, Kapuas Hulu District is to re-plan the roundabout following the Roundabout Planning Guidelines for Level Crossings (2004). The re-planning of the roundabout can be seen in Figure 3.

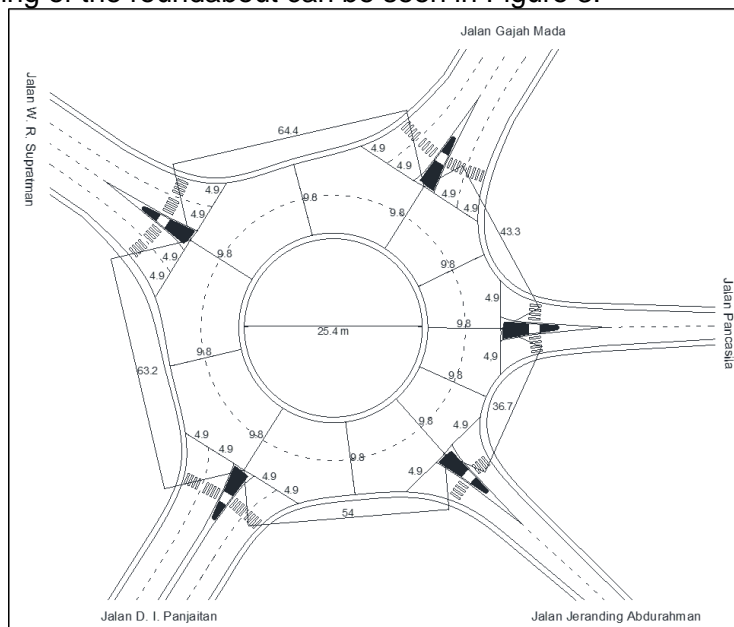


Fig 3. Roundabout Re-planning

The results of the braid section geometric parameters calculation of the roundabout re-planning solution for 2028 projection conditions can be seen in Table 13.

Table 13. Braid Section Geometric Parameter Calculation Results of The Re-planning Solution for 2028 Projection Condition

No	Braid Section	Entry Width		Average Entry Width	Braid Width	WE/WW	Braid Length	WW/LW
		Approach 1 (m)	Approach 2 (m)	WE (m)	WW (m)		LW (m)	
1	AB	9,8	9,8	9,8	9,8	1,000	63,2	0,155
2	BC	9,8	9,8	9,8	9,8	1,000	64,4	0,152
3	CD	9,8	9,8	9,8	9,8	1,000	43,3	0,226
4	DE	4,9	9,8	7,35	9,8	0,750	36,7	0,267
5	EA	4,9	9,8	7,35	9,8	0,750	54	0,181

The results of the braid section capacity calculation of the re-planning solution for 2028 projection conditions can be seen in Table 14.

Table 14. Braid Section Capacity Calculation Results of The Re-planning Solution for 2028 Projection Condition

No	Braid Section	WW Factor	WE/WW Factor	PW Factor	WW/LW Factor	Base Capacity C0	Adjustment Factor		Capacity C smp/hour
							City Size	Street Environment (FRSU)	
1	AB	2623,782	2,828	0,523	0,771	2995,965	0,88	0,95	2504,627
2	BC	2623,782	2,828	0,526	0,775	3026,440	0,88	0,95	2530,104
3	CD	2623,782	2,828	0,558	0,693	2867,780	0,88	0,95	2397,464
4	DE	2623,782	2,315	0,682	0,653	2707,438	0,88	0,95	2263,418
5	EA	2623,782	2,315	0,449	0,741	2021,264	0,88	0,95	1689,777

The results of the braid section traffic behaviour calculation of the re-planning solution for 2028 projection conditions can be seen in Table 15.

Table 15. Braid Section Traffic Behaviour Calculation Result of The Re-Planning Solution For 2028 Projection Condition

No	Braid Section	Flow of Braid Section Q smp/hour	Degree of Saturation DS	Traffic Delay DT sec/smp	Total Traffic Delay DTOT = Q x DT	Queue Opportuniy QP%
1	AB	860	0,34	1,6	1385,486	3 - 7
2	BC	924	0,37	1,7	1581,870	4 - 8
3	CD	875	0,36	1,7	1497,924	4 - 8
4	DE	845	0,37	1,8	1480,578	4 - 8
5	EA	906	0,54	2,5	2277,786	7 - 15
6	Degree of Saturation (DS)					0,54
7	Average Traffic Delay (DTR) sec/smp					5,1
8	Average Roundabout Delay DR (DTR+4) sec/smp					9,07
9	Roundabout Queue Opportunity QPR%					7 - 15

The results of the VISSIM software simulation of the roundabout re-planning solution for 2028 projection conditions can be seen in Table 16.

Table 16. VISSIM Simulation Results of The Roundabout Re-planning for 2028 Projection Condition

Code	From	Towards	Queue Length (m)	Maximum Queue Length (m)	Delay (sec)	Level of Service
A1	D. I. Panjaitan	D. I. Panjaitan	3,53	25,87	7,96	A
A2	D. I. Panjaitan	W. R. Supratman	3,53	25,87	8,78	A
A3	D. I. Panjaitan	Gajah Mada	3,53	25,87	7,15	A
A4	D. I. Panjaitan	Pancasila	3,53	25,87	7,90	A
A5	D. I. Panjaitan	Jeranding Abdurahman	3,53	25,87	5,86	A
B1	W. R. Supratman	D. I. Panjaitan	2,07	16,42	14,58	B
B2	W. R. Supratman	W. R. Supratman	2,07	16,42	0,00	A
B3	W. R. Supratman	Gajah Mada	2,07	16,42	16,27	C
B4	W. R. Supratman	Pancasila	2,07	16,42	11,27	B
B5	W. R. Supratman	Jeranding Abdurahman	2,07	16,42	14,48	B
C1	Gajah Mada	D. I. Panjaitan	2,78	12,45	13,77	B
C2	Gajah Mada	W. R. Supratman	2,78	12,45	12,23	B
C3	Gajah Mada	Gajah Mada	2,78	12,45	0,00	A
C4	Gajah Mada	Pancasila	2,78	12,45	9,12	A
C5	Gajah Mada	Jeranding Abdurahman	2,78	12,45	14,74	B
D1	Pancasila	D. I. Panjaitan	4,04	27,62	16,54	C
D2	Pancasila	W. R. Supratman	4,04	27,62	20,05	C
D3	Pancasila	Gajah Mada	4,04	27,62	13,51	B
D4	Pancasila	Pancasila	4,04	27,62	0,00	A
D5	Pancasila	Jeranding Abdurahman	4,04	27,62	10,60	B
E1	Jeranding Abdurahman	D. I. Panjaitan	3,71	23,36	26,23	D
E2	Jeranding Abdurahman	W. R. Supratman	3,71	23,36	16,38	C
E3	Jeranding Abdurahman	Gajah Mada	3,71	23,36	27,45	D
E4	Jeranding Abdurahman	Pancasila	3,71	23,36	17,45	C
E5	Jeranding Abdurahman	Jeranding Abdurahman	3,71	23,36	0,00	A
Roundabout			3,22	27,62	13,14	B

The results of the braid section traffic behaviour calculation of the re-planning solution for 2033 projection conditions can be seen in Table 17.

Table 17. Braid Section Traffic Behaviour Calculation Result Of The Re-Planning Solution For 2033 Projection Condition

No	Braid Section	Flow of Braid Section Q smp/hour	Degree of Saturation DS	Traffic Delay DT sec/smp	Total Traffic Delay DTOT = Q x DT	Queue Opportuniy QP%
1	AB	1018	0,41	1,9	1941,902	4 - 9
2	BC	1094	0,43	2,0	2217,994	5 - 10
3	CD	1036	0,43	2,0	2099,647	5 - 10
4	DE	1001	0,44	2,1	2077,464	5 - 10
5	EA	1073	0,63	3,1	3368,504	10 - 22
6	Degree of Saturation (DS)					0,63
7	Average Traffic Delay (DTR) sec/smp					6,1
8	Average Roundabout Delay DR (DTR+4) sec/smp					10,09
9	Roundabout Queue Opportunity QPR%					10 - 22

The results of the VISSIM software simulation of the roundabout re-planning solution for 2033 projection conditions can be seen in Table 18.

Table 18. VISSIM Simulation Results of The Roundabout Re-planning for 2033 Projection Condition

Code	From	Towards	Queue Length (m)	Maximum Queue Length (m)	Delay (sec)	Level of Service
A1	D. I. Panjaitan	D. I. Panjaitan	9,34	49,41	9,50	B
A2	D. I. Panjaitan	W. R. Supratman	9,34	49,41	8,30	B
A3	D. I. Panjaitan	Gajah Mada	9,34	49,41	9,31	B
A4	D. I. Panjaitan	Pancasila	9,34	49,41	8,03	B
A5	D. I. Panjaitan	Jeranding Abdurahman	9,34	49,41	8,28	A
B1	W. R. Supratman	D. I. Panjaitan	4,53	16,23	15,94	C
B2	W. R. Supratman	W. R. Supratman	4,53	16,23	19,28	F
B3	W. R. Supratman	Gajah Mada	4,53	16,23	6,84	B
B4	W. R. Supratman	Pancasila	4,53	16,23	9,75	C
B5	W. R. Supratman	Jeranding Abdurahman	4,53	16,23	12,28	B
C1	Gajah Mada	D. I. Panjaitan	2,81	15,38	14,21	C
C2	Gajah Mada	W. R. Supratman	2,81	15,38	18,04	E
C3	Gajah Mada	Gajah Mada	2,81	15,38	0,00	A
C4	Gajah Mada	Pancasila	2,81	15,38	12,29	A
C5	Gajah Mada	Jeranding Abdurahman	2,81	15,38	10,30	B
D1	Pancasila	D. I. Panjaitan	5,19	19,94	15,28	F
D2	Pancasila	W. R. Supratman	5,19	19,94	20,14	D
D3	Pancasila	Gajah Mada	5,19	19,94	38,70	D
D4	Pancasila	Pancasila	5,19	19,94	0,00	A
D5	Pancasila	Jeranding Abdurahman	5,19	19,94	13,93	C
E1	Jeranding Abdurahman	D. I. Panjaitan	10,61	33,54	33,98	D
E2	Jeranding Abdurahman	W. R. Supratman	10,61	33,54	54,27	D
E3	Jeranding Abdurahman	Gajah Mada	10,61	33,54	30,26	D
E4	Jeranding Abdurahman	Pancasila	10,61	33,54	33,92	D
E5	Jeranding Abdurahman	Jeranding Abdurahman	10,61	33,54	0,00	A
	Roundabout		6,49	49,41	15,60	C

3.7 Classification of Performance Analysis Results of Existing Conditions, Projection Conditions in 2028 and 2033

After the analysis for the roundabout re-planning, the following class classification on the highest degree of saturation value and average delay for existing conditions, projection conditions in 2028 and 2033, based on MKJI 1997 calculations can be seen in Table 19.

Table 19. Class Classification of Degree of Saturation and Average Delay of Roundabout Re-planning Solution for 2028 and 2033 Projection Condition

No	Classification	Result	Level of Service
1	DSR Projection Result (2028)	0,54	C
	DSR Projection Result (2033)	0,63	C
2	DT _R (sec) Projection Result (2028)	9,07	B
	DT _R (sec) Projection Result (2033)	10,09	B

After simulation for Roundabout Re-planning Solution in VISSIM software. Class classification on the value of delay based on simulation results for 2028 and 2033 projection conditions, can be seen in Table 20.

Table 20. Class Classification of Delays Based on VISSIM Software Simulations of Roundabout Re-planning Solution for 2028 and 2033 Projection Condition

No	Classification	Result	Level of Service
1	Delay Projection result (2028)	13,14	B
2	Delay Projection result (2033)	15,60	C

3.8 Comparison of MKJI 1997 and VISSIM Software Results

3.8.1 Comparison of MKJI 1997 Queue Opportunity Results and VISSIM Software Queue Lengths

The results of queuing opportunities in the braided section using MKJI 1997 are obtained in units of per cent, while the results of queue length using VISSIM software are obtained in units of metres, which can be seen in Table 21.

Table 21. Comparison of MKJI 1997 Queue Opportunity and VISSIM Software Queue Length Results

Condition	Braid Section	MKJI 1997	Software VISSIM
		Queue Opportunity QP%	Queue Length (m)
Existing 2023	AB	3 - 7	4,02
	BC	5 - 9	2,67
	CD	5 - 11	33,54
	DE	5 - 9	4,69
	EA	10 - 21	15,93
Projection 2028	AB	4 - 9	10,61
	BC	6 - 13	2,02
	CD	7 - 15	13,68
	DE	6 - 13	57,85
	EA	15 - 32	50,7
Projection 2033	AB	6 - 12	21,17
	BC	8 - 18	27,45
	CD	10 - 23	70,6
	DE	9 - 19	26,5
	EA	24 - 51	95,69
Roundabout Re-planning Solution Projection 2028	AB	3 - 7	3,53
	BC	4 - 8	2,07
	CD	4 - 8	2,78
	DE	4 - 8	4,04
Roundabout Re-planning Solution Projection 2033	EA	7 - 15	3,71
	AB	4 - 9	9,34
	BC	5 - 10	4,53
	CD	5 - 10	2,81
	DE	5 - 10	5,19
	EA	10 - 22	10,61

3.8.2 Comparison of MKJI 1997 and VISSIM Software Delay Results

A comparison of delay results using MKJI 1997 and VISSIM software can be seen in Table 22.

Table 22. Comparison of Delay Results Using MKJI 1997 And VISSIM Software

Condition	Delay (sec)		Level Of Service	
	MKJI 1997	Software VISSIM	MKJI 1997	Software VISSIM
Existing 2023	9,78	17,81	B	C
Projection 2028	11,25	28,21	B	D
Projection 2033	13,75	48,31	B	E
Roundabout Re-planning Solution Projection 2028	9,07	13,14	B	B
Roundabout Re-planning Solution Projection 2033	10,09	15,6	B	C

4. Conclusion

From the research that has been done, it can be concluded that based on the Indonesian Road Capacity Manual (1997), in the existing conditions of 2023, the performance of the roundabout has the highest degree of saturation of 0.62, and in the projection conditions of 2028 and 2033 the highest

degree of saturation is 0.73 and 0.86. Furthermore, by re-planning the roundabout using the results of the 2028 and 2033 projections, the highest degree of saturation is obtained at 0.54 and 0.63. So, there is a decrease in the degree of saturation value in 2028 and 2033. The results of the roundabout performance analysis using VISSIM software show that in 2023, the roundabout performance has a roundabout delay value of 17.81 seconds, while in the projected conditions of 2028 and 2033, it is 28.21 seconds and 48.28 seconds, respectively. By re-planning the roundabout, the delay value significantly decreases, namely in 2028 to 13.14 seconds and in 2033 to 19.71 seconds. Thus, planning a roundabout by following the Roundabout Planning Guidelines for Level Crossings (2004) has a better level of service.

5. Acknowledgement

First, I would like to thank my beloved parents, who always supported and prayed to complete this research. I would also like to thank Mrs. Dr. Elsa Tri Mukti, S.T., M.T., Mrs. S. Nurlaily Kadarini, S.T., M.T., IPM., Mr. Heri Azwansyah S.T., M.T., IPM, and Mr. Dr. Said S.T., M.T., who have provided very useful knowledge and helped during the process of completing this final project. Then I also thank my girlfriend and my friends who always provide support and help so that I have the enthusiasm to complete this final project. Finally, I would like to thank the Jurnal Teknik Sipil UNTAN (JTS) team for being willing to publish the results of my research. Hopefully, the results of this research can be useful for many people, especially in research on roundabout performance analysis and evaluation.

6. Author's Note

The author declares that this article is an original work and does not contain plagiarism of any research, because it has passed the examination to obtain a bachelor of engineering degree at the Faculty of Engineering, Tanjungpura University, on December 22, 2023.

7. References

- BPS Kabupaten Kapuas Hulu. 2023. *Kabupaten Kapuas Hulu Dalam Angka 2023*. <https://kapuashulukab.bps.go.id/publication/2023/02/28/28a488c2453c023b16d2fe03/kabupaten-kapuashulu-dalam-angka-2023.html>
- Departemen Peremukiman dan Prasarana Wilayah. 2004. *Perencanaan Bundaran untuk Pesimpangan Sebidang*. Jakarta: Departemen Peremukiman dan Prasarana Wilayah.
- Dharmawan, W. I. 2016. *Analisa Kinerja Bundaran Menggunakan Metode Manual Kapasitas Jalan Indonesia (MKJI) (Studi Kasus : Bundaran Radin Inten Bandar Lampung)*. Tugas Akhir Program Studi Teknik Sipil, Fakultas Teknik Universitas Malahayati, Lampung.
- Direktorat Jenderal Bina Marga. (1997). *Manual Kapasitas Jalan Indonesia (MKJI 1997)*. Jakarta: Departemen Pekerjaan Umum.
- Irawan, M. Z. dan Putri, N. H. (2015). *Kalibrasi VISSIM Untuk Mikrosimulasi Arus Lalu Lintas Tercampur pada Simpang Bersinyal (Studi Kasus : Simpang Tugu Yogyakarta)*. Jurnal Penelitian Transportasi Multimoda.
- Jamanda, F. (2016). *Analisis Bundaran Jalan Arteri Supadio dan Jalan Mayor Alianyang*. Tugas Akhir Program Studi Teknik Sipil, Fakultas Teknik, Universitas Tanjungpura, Pontianak.
- Kartika, S. W. (2016). *Analisis dan Evaluasi Kinerja Bundaran Smp Negeri 1 Pontianak*. Tugas Akhir Program Studi Teknik Sipil, Fakultas Teknik, Universitas Tanjungpura, Pontianak.
- Khisty, J. C. dan Lall, B. K. (2005). *Dasar-Dasar Rekayasa Transportasi. Jilid 1*. Diterjemahkan oleh Fidel Miro. Jakarta: Erlangga.
- Lo, O. C., Mukti, E. T., & Juniardi, F. (2023). *Intersection Arrangement Design on Sultan Hamid II Road-Gusti Situt Mahmud Road–28 Oktober Road–Selat Panjang Road Pontianak Due to the Operation of the Landak Parallel Bridge*. Jurnal Teknik Sipil, 23(4), 19-25.
- Osei, K. K., Adams, C. A., Ackaah, W., & Oliver-Commey, Y. (2021). *Signalization options to improve capacity and delay at roundabouts through microsimulation approach: A case*

- study on arterial roadways in Ghana.** Journal of Traffic and Transportation Engineering (English Edition), 8(1), 70-82.
- Pemayun, M. I. C. (2015). **Analisis Kinerja Ruas Jalan Diponegoro Akibat Bangkitan Perjalanan Sdn 5 Pendungan.** Tugas Akhir Program Studi Teknik Sipil, Fakultas Teknik Universitas Udayanan, Bali.
- Peraturan Menteri Perhubungan Republik Indonesia Nomor PM 96 Tahun 2015. **Pedoman Pelaksanaan Kegiatan Manajemen dan Rekayasa Lalu Lintas.** Jakarta: Menteri Perhubungan Republik Indonesia.
- Prabowo, S. (2021). **Evaluasi Kinerja Bundaran Ale Ale Di Kota Ketapang.** Tugas Akhir Program Studi Teknik Sipil, Fakultas Teknik, Universitas Tanjungpura, Pontianak.
- Priyatmoko, T. N.(2018). **Analisis Dan Evaluasi Bundaran Tugu Jam Di Kota Sintang.** Tugas Akhir Program Studi Teknik Sipil, Fakultas Teknik, Universitas Tanjungpura, Pontianak.
- PTV VISION. (2011). **Vissim 5.30-05 User Manual.** PTV AG. Karlsruhe, Germany.
- Putra, A. D. (2019). **Analisis Kinerja Bundaran Leuwigajah Kota Cimahi.** Tugas Akhir Program Studi Teknik Sipil, Fakultas Teknik Sipil, Institut Teknologi Nasional, Bandung.
- Putro, B. T. B. A. (2016). **Analisis Kinerja Bundaran Jombor Yogyakarta (The Analysis Of Roundabout Performance In Jombor Yogyakarta).** Tugas Akhir Program Studi Teknik Sipil, Fakultas Teknik Sipil dan Perencanaan, Universitas Islam Indonesia, Yogyakarta.
- Risdiyanto. (2014). **Rekayasa & Manajemen Lalu Lintas Teori dan Aplikasi.** Yogyakarta: PT Leutika Nouvalitera.
- Romadhona, J. P., Ikhsan, N. T., dan Prasetyo, D. (2019). **Aplikasi Permodelan PTV VISSIM 9.0.** Yogyakarta : UII Press.
- Siyoto, Sandu., dan Ali Sodik. (2015). **Dasar Metodologi Penelitian.** Yogyakarta: Literasi Media Publishing.
- Strada, E. (2021). **Analisis Kinerja Bundaran Joeang di Kota Palangkaraya Provinsi Kalimantan Tengah.** Tugas Akhir Program Studi Teknik Sipil, Fakultas Teknik, Universitas Palangkaraya, Palangkaraya.
- Sukmawati, I. (2022). **Analisis Dan Evaluasi Bundaran 1001 Ai Kota Singkawang.** Tugas Akhir Program Studi Teknik Sipil, Fakultas Teknik, Universitas Tanjungpura, Pontianak.